Miller**GRAY**

May 2, 2023

delivered via email to kfoutz@marblefallstx.gov

Kim Foutz Director of Development Services City of Marble Falls 801 W 4th Street Marble Falls, Texas 78654

Re: Water & Wastewater Assessment (WA-223) Version 2 City of Marble Falls, Texas

Black Jack Marble 56, Jackson Road, Full-Purpose Jurisdiction

Dear Ms. Foutz,

Miller Gray (MG) has reviewed the water and wastewater submittal documents for a development known as "Black Jack Marble 56" (BJ56), which is located east of Jackson Road and south of Panther Hollow Drive in the Full-Purpose Jurisdiction of the City of Marble Falls. The City tasked MG with preparing a water and wastewater assessment of the area for general conformance with City and State Codes and to verify impacts to the water and wastewater systems. Please see below for further description of the project assessment results.



Miller Gray LLC • 7320 N Mopac Expy, Suite 203 • Austin, Texas, USA 78731 (512) 861-5300 • www.miller-gray.com • Project No. 01109-223

1. Background Information

The water and wastewater assessment for the BJ56 development utilizes applicant supplied information with the documents listed below included as attachments.

- Attachment A Service Request Form
- Attachment B Land Survey
- Attachment C Conceptual Land Plan

2. Project Details

The estimated number of LUEs within the BJ56 development classified by type of use are summarized below.

- Condo/Apartment Unit Low Density:
- Office/Amenity for Low Density:
- Condo/Apartment Unit High Density:
- Office/Amenity for High Density:

327.6 LUEs (468 units @ 0.7 LUE per unit)

- 2.5 LUEs (assumed for typical Condo/Apartment)
- 207.5 LUEs (415 units @ 0.5 LUE per unit)
- 2.5 LUEs (assumed for typical Condo/Apartment) 540 LUEs on 57.00 acres

• Total LUEs:

The range of existing ground elevations within the limits of the BJ56 development are as follows:

- Lowest Ground Elevation: ~860 feet
- Highest Ground Elevation: ~965 feet

3. Water Assessment

The BJ56 development does not currently receive water service and requires connections to the nearest available water system with adequate pressure, flow, and storage. MG prepared an estimate of water demand along with an evaluation of water supply and associated impacts as noted below. A map showing the subject tract along with the existing water system is shown in Attachment D – Water Service Area Map.

3.1 Existing Pressure Planes & Storage

The City has two existing pressure planes that could potentially serve the BJ56 development. A summary of the pressure planes and their associated Hydraulic Grade Lines (HGLs) are outlined below.

Gateway Park Pressure Plane

The Gateway Park Pressure Plane (GP-PP) is served by the Gateway Park Booster Pump Station (GP-BPS) and the Gateway Park Standpipe (GP-SP).

- Existing system with a typical HGL of 1,020 ft.
- Serves ground elevations 736 ft. to 932 ft.
- Rated pumping capacity of 500 gpm per TCEQ §290.45.
- Rated pumping capacity of 833 LUEs (at 0.6 gpm/LUE).

The existing GP-SP has an elevated storage volume of 47,500 gallons with all storage capacity allocated to existing customers as noted below.

- Existing Storage Capacity: 47,500 gallons (elevated)
- Existing Storage Capacity: 237 LUEs (storage rate of 200 gal/LUE)

Flatrock Pressure Plane

The Flatrock Pressure Plane (FL-PP) is served by the Flatrock Booster Pump Station (FL-BPS) and the Flatrock Elevated Storage Tank (FL-EST).

- Existing system with a typical HGL of 1,271 ft.
- Serves ground elevations 932 ft. to 1,187 ft.
- Rated pumping capacity of 750 gpm per TCEQ §290.45.
- Rated pumping capacity of 1,250 LUEs (at 0.6 gpm/LUE).

The existing FL-EST has an elevated storage volume of 1,000,000 gallons with all storage capacity allocated to existing customers as noted below.

- Existing Storage Capacity: 1,000,000 gallons (elevated)
- Existing Storage Capacity: 5,000 LUEs (storage rate of 200 gal/LUE)

3.2 Existing Distribution System

The BJ56 development is required to be supplied by FL-PP and the associated 16 inch Flatrock Transmission Main along the western side of US 281 between Panther Hollow Drive and Rocky Road.

The FL-PP pressures in this area require Pressure Reducing Valves (PRVs) which create a reduced zone in the vicinity of the BJ56 development. The reduced zone has not yet been established; however, it is proposed to be created by a series of three PRVs with one installed by the Little Panther Apartments development (City Assessment WA-220) and two installed by the Roper West development (City Assessment WA-205).

The three PRVs are expected to connect to the Flatrock Transmission Main and supply the existing 12 inch South Water Main as shown in Attachment D. The characteristics of the reduced zone are as follows:

- Proposed system with a typical HGL of 1,065 ft.
- Serves ground elevations 755 ft. to 985 ft.

3.3 Project Water Demand

The consumer water demand for the BJ56 development is estimated as noted below along with a calculation provided in Attachment E – Water Demand Calculations.

Total Project

- ADD: 113.4 gpm / 0.16 mgd (average daily demand, calculated)
- PDD: 226.8 gpm / 0.33 mgd (peak day demand, calculated)
- PHD: 302.4 gpm / 0.44 mgd (peak hour demand, calculated)
- FFD: 1,500.0 gpm / 2.16 mgd (fire flow demand, assumed)
- EMD: 1,726.8 gpm / 2.49 mgd (emergency demand, calculated)

3.4 Project Water Storage

The water system is required to provide a total storage volume of 200 gal/LUE per TCEQ 290.45(b)(2)(E) and a minimum elevated storage volume of 100 gal/LUE per TCEQ 290.45(b)(2)(G).

- LUEs: 540
- Min. Elevated Storage: 54,000 gallons
- Total Storage: 108,000 gallons
- All storage volumes for the GP-PP and FL-PP are allocated to existing users.

3.5 Project Water Pumping

MG evaluated the FL-BPS pumping capacity to determine the ability to serve the BJ56 development. The evaluation assumes that all storage will be elevated, resulting in the requirement of: "For systems which provide an elevated storage capacity of 200 gallons per connection, two service pumps with a minimum combined capacity of 0.6 gpm per connection are required at each pump station or pressure plane" per TCEQ 290.45(b)(2)(F). A summary of pumping requirements is outlined below.

• Total pumping requirement for the BJ56 development is an additional 324 gpm of rated capacity serving 540 LUEs.

3.5 Project Water Main Connections

MG evaluated new water main connections to the proposed FL-PP reduced zone and considered water piping improvements necessary to serve the BJ56 development. The evaluation considered primary connections to the existing 12 inch mains serving the Roper West development as shown on Attachment D.

3.6 Project Water Pressures

MG evaluated peak hour demand (PHD) and emergency demand (EMD) impacts to the affected water systems and calculated pressures expected within the project. The minimum allowable pressures at all points within the distribution network are 35 psi for PHD and 20 psi for EMD, per TCEQ §290.44(d). All analyses assume that minimum storage is provided in the FL-PP and utilizes the reduced zone typical HGL.

- PHD– MG calculated PHD pressures for the supply scenario described above ranging from 43 psi to 88 psi at all model junctions within the BJ56 development.
- EMD MG calculated EMD total available flows with no residual pressures of less than 20 psi.

3.7 Water Summary & Recommendations

Recommendations for improvements to the water system based on impacts from the BJ56 development are outlined as follows:

Part I: Piping Connections

- The Applicant shall provide a minimum of two 12 inch connections to the existing 12 inch mains as shown on Attachment D.
- The Applicant shall route the 12 inch mains into the development and internally loop.
- The Applicant shall acquire any necessary easements to connect to the Flatrock Transmission main.

Part II: Pumping Improvements

- MG recommends the installation of new pumping capacity to serve the GP-PP with a preliminary cost of \$1,500,000 and rated capacity of 5,000 gpm (\$300/gpm).
- MG recommends that the Applicant contribute their pro-rata share of the upgraded pumping cost totaling \$90,600 (302 gpm @ \$300/gpm).

Part III: Elevated Storage Improvements

- MG recommends the installation of additional storage capacity to serve the area near BJ56, and the City is currently evaluating a new Elevated Storage Tank with a preliminary cost of \$5,000,000 and storage volume of 1,000,000 gallons (\$5/gal).
- MG recommends that the Applicant contribute their pro-rata share of the upgraded storage cost totaling \$540,000 (108,000 gal @ \$5/gal).

4. Wastewater Assessment

The BJ56 development does not currently receive wastewater service and requires connections to the nearest available system with adequate capacity. MG prepared an estimate of wastewater demand along with an evaluation of wastewater capacity and associated impacts as noted below.

A map showing the subject tract along with the existing wastewater system is shown in Attachment F – Wastewater Service Area Map. Please note that this assessment does not comprehensively examine wastewater availability in the system for the areas north of Lake Marble Falls.

4.1 Existing Gravity Wastewater System

The City has two wastewater collection systems currently in service near the development. A general description of the two gravity mains is provided below.

Panther Hollow Gravity Main

- The Panther Hollow Gravity Main (PHGM) consists of a 8 inch PVC gravity main, approximately 813 linear feet long, with a downstream limit at the Panther Hollow Lift Station and an upstream limit near the southern terminus of Panther Hollow Drive.
- The PHGM generally has a total capacity of 573 gpm and maximum allowable capacity of 487 gpm when considering that PWWF shall be limited to 85% of total capacity; however, slope conditions restrict capacity at two bottlenecks along the pipe. The PHGM bottlenecks and capacities are described as follows:

Bottleneck No. 1

- Bottleneck No. 1 occurs between Stations 0+00 and 0+20 where the pipe flattens in slope for a distance of 20 linear feet restricting flow to a total capacity of 352.7 gpm.
- Bottleneck No. 1 causes this portion of the main to have a *maximum allowable capacity* of 299.8 gpm.

Bottleneck No. 2

- Bottleneck No. 2 occurs between Stations 3+12 and 4+37 where the pipe flattens in slope for a distance of 125 linear feet restricting flow to a total capacity of 286.4 gpm.
- Bottleneck No. 2 causes this portion of the main to have a *maximum allowable capacity* of 243.4 gpm.

Existing Development & Capacity

• Existing (and recently authorized) development served by the PHGM is estimated to consume *157.6 gpm of capacity*.

Rocky Road Gravity Main

- The Rocky Road Gravity Main (RRGM) consists of varied diameter PVC gravity main (6 and 8 inch), approximately 2,111 linear feet long, with a downstream limit at the Panther Hollow Lift Station and an upstream limit at WorldMark Resort Marble Falls.
- The 8 inch portion of the RRGM near the PHLS generally has a total capacity of 352.8 gpm and maximum allowable capacity of 299.9 gpm when considering that PWWF shall be limited to 85% of total capacity.
- Existing development served by the RRGM is estimated to consume 22.4 gpm of capacity.

4.2 Existing Lift Station System

The PHGM and RRGM flow into the existing Panther Hollow Lift Station as noted above. The characteristics of the lift station are as follows:

Existing Panther Hollow Lift Station

- Well Diameter = 9 ft.
- Well Material = Fiberglass
- Well Depth = 15 ft. (lid to invert)
- Pump Quantity = 2 (duplex pump configuration)
- Pump Capacity = 197 gpm (firm pumping capacity)
- Force Main Diameter = 4 in. PVC
- Force Main Velocity = 5.0 fps
- Force Main Pressure = 108 psi

4.3 Project Wastewater Demand

The consumer wastewater demand for the BJ56 development is estimated using Applicant supplied LUEs and land uses. A summary of wastewater demand is noted below along with a wastewater demand calculation shown in Attachment G.

- ADWF: 90.2 gpm / 0.13 mgd (average dry weather flow)
- PDWF: 225.2 gpm / 0.32 mgd (peak dry weather flow)
- I&I: 29.7 gpm / 0.04 mgd (inflow & infiltration)
- PWWF: 255.1 gpm / 0.37 mgd (peak wet weather flow)

4.4 Project Wastewater Connections

The wastewater assessment evaluated connecting to the RRGM given the capacity restrictions on the PHGM and determined the following:

Gravity Main Piping Connection

- The RRGM has a maximum allowable capacity of 299.9 gpm.
- Existing development served by the RRGM is estimated to consume 22.4 gpm of capacity.
- The RRGM can accommodate all 540 LUEs (251.6 gpm) as noted above.

Upgraded Panther Hollow Lift Station

The Panther Hollow Lift Station requires upgrades to serve the BJ56 development and must consider flows from existing and recently proposed adjacent developments. The upgraded lift station will need to convey the following Peak Wet Weather Flows at a minimum:

- Existing: 197.0 gpm
- Adj. Developments: 301.4 gpm
- BJ56: 251.6 gpm
- Total: 750.0 gpm

Upgraded Panther Hollow Force Main

The Panther Hollow Force Main requires upgrades to serve the BJ56 development and must consider flows from existing and recently proposed adjacent developments. The upgraded force main will need to convey the Peak Wet Weather Flows noted above.

• The Panther Hollow Lift Station requires a 12 inch force main when accounting for the Peak Wet Weather Flows noted above.

4.5 Wastewater Recommendations

Recommendations for improvements to the wastewater system based on the impacts from the BJ56 development are outlined below and on the annotated System Map shown in Attachment F.

Piping Connections

- The Applicant shall connect to the existing 8 inch RRGM at an existing manhole and acquire any necessary easements.
- The proposed BJ56 main shall be an 8 inch gravity main and dedicated to the public.

Lift Station

- MG recommends the installation of new pumping capacity at the Panther Hollow Lift Station, with a rated pumping capacity of 750 gpm at a preliminary cost of \$450,000 (\$600/gpm).
- MG recommends that the Applicant contribute their pro-rata share of the new pumping capacity cost totaling \$150,960 (251.6 gpm @ \$600/gpm).

Force Main

- MG recommends the installation of a new 12 inch force main for a distance of ~2,070 linear feet with a capacity of 750 gpm to serve the upgraded Panther Hollow Lift Station, at a preliminary cost of \$414,000 (\$552/gpm).
- MG recommends that the Applicant contribute their pro-rata share of the new force main capacity cost totaling \$138,883 (251.6 gpm @ \$552/gpm).

Please review this summary letter and should you have any questions, please feel free to contact our office.

Sincerely,

MillerGRAY TBPE Firm Reg. No. F-16302

Samuel C. Shorter, P.E. *Senior Engineer*

Cc: Dale Gray, P.E. – Miller Gray

- Attachment A: Service Request Form
- Attachment B: Land Survey
- Attachment C: Conceptual Plan
- Attachment D: Water Service Area Map
- Attachment E: Water Demand Summary
- Attachment F: Wastewater Service Area Map
- Attachment G: Wastewater Demand Summary





City of Marble Falls – Water & Wastewater Service Request

Part I – Overview

- 1. The Water & Wastewater Service Request form is intended for developments of four (4) or more living unit equivalents (LUEs) or as prescribed by the City of Marble Falls.
- 2. The Applicant shall provide all necessary information to assess water & wastewater service, and is made aware that additional information may be required as part of the request.
- 3. At a minimum, the Applicant shall supply a basic plan (i.e., Conceptual, Land, or Preliminary Plan) showing existing water & wastewater utility lines along with all proposed lines and connections (including pipe sizes and materials).
- 4. For multi-building developments other than single-family residential, the Applicant shall provide square footage and usage type for each building.
- 5. The Applicant may attach additional sheets information if necessary.

Project Name	Black Jack Marble 56	
Site Address(es)	Jackson Rd	
Tax Parcel ID Number(s)	119649	
Current Legal Owner	Black Jack Marble Falls 58, LLC	
Total Land Area (acres)	57	

Part II – Background Information

Part III – Service Needs

Request for Water (y/n)	Y
Request for Wastewater (y/n)	Y
Request for Reclaimed Water (y/n)	N
Request for Fire Flow (y/n)	Y
Lowest Land Elevation Served	860
Highest Land Elevation Served	965

Part IV – Plan Information

Basic Plan Attached (y/n?)	Y
Plan Type (Conceptual, Land,	Concept
Detailed Plan Attached (y/n?)	N
Plan Type (Site, Construction, Utility?)	



Part V - Land Us	e Information
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Single Family Residence, Modular Home, Mobile Home		(number of units)
Duplex		(number of duplexes)
Triplex, Fourplex		(number of units)
Condo / Apartment Unit (less than 24 units/ac)	468	(number of units)
Condo / Apartment Unit (24 units/ac or greater)	415	(number of units)
Hotel or Motel Room		(number of rooms)
Office		(total square feet)
Warehouse		(total square feet)
Retail, Shopping Center		(total square feet)
Restaurant, Cafeteria		(total square feet)
Hospital		(number of beds)
Rest Home		(number of beds)
Church (Worship services only)		(number of seats)
High School / Middle School (incl. Gym & Cafeteria)		(number of students)
Elementary School (incl. Gym & Cafeteria)		(number of students)
Other (Specify)		(number of)

*****END*****

Attachment B - Land Survey



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Attachment C - Conceptual Plan







Attachment E - Water Demand Calculations

Project:	Black Jack W&WW Assessment
Job No.:	01109-223
Date:	4/21/2023
By:	SCS/MG
Title:	Water Demand Summary

WATER DEMAND SUMMARY						
LandLUEsAreaWater ADDWater PDDWater PHD% of ToUnit(no.)(ac)(gpm)(gpm)(gpm)Flow						% of Total Flow
Condo/Apts.	540	57.0	113.4	226.8	302.4	100.0%
Totals	540	57.0	113.4	226.8	302.4	

Water Demand Flow Rates				
Item	Qty	Units		
ADD 0.210 gpm				
PDD	0.420	gpm/LUE		
PHD	0.560	gpm/LUE		

Water Storage					
Item	Qty	Units			
LUEs 540					
Min. Elev.	54,000	gallons			
Total	108,000	gallons			

Emer. Demand Flow Rates					
Item	Qty	Units			
Fire Flow	1,500.0	gpm			
Total PDD	226.8	gpm			
Total EMD	1,726.8	gpm			



Attachment G - Wastewater Demand Calculations

Project:Black Jack W&WW AssessmentJob No.:01109-223Date:4/21/2023By:SCS/MGTitle:Wastewater Demand Summary

WASTEWATER DEMAND SUMMARY							
Land Unit	LUEs (no.)	Area (ac)	ADWF (gpm)	PDWF (gpm)	I&I (gpm)	PWWF (gpm)	% of Total Flow
Condo/Apts.	540	57.0	90.2	225.5	29.7	255.1	100.0%
Totals	540	57.0	90.2	225.5	29.7	255.1	•

Wastewater Flow Rates					
Item	Qty	Units			
ADWF	240	gpd/LUE			
ADWF	0.167	gpm/LUE			
PDWF	600	gpd/LUE			
PDWF	0.417	gpm/LUE			
I&I	750	gpd/acre			
I&I	0.521	gpm/acre			